

PG	X-LOC	REACT	SIZE	REQ'D
1	0-1-12	1191	3.50"	1.50"
2	25-2-4	1376	3.50"	1.50"

TC	FORCE	AXL	END	CSI
1	-486	.04	.20	.24
2	-379	.04	.12	.16
3	-582	.06	.14	.20
4	-819	.05	.37	.42
5	-1304	.07	.37	.44
6	-1484	.07	.36	.43
7	-1723	.07	.36	.43

TC	FORCE	AXL	END	CSI
1	658	.08	.05	.12
2	442	.04	.31	.35
3	972	.08	.31	.39
4	972	.08	.45	.53
5	1348	.11	.45	.56

WEB	FORCE	CSI	WEB	FORCE	CSI
1-9	-1151	.58	4-11	157	.02
1-10	856	.62	5-11	-656	.35
2-10	176	.28	5-13	501	.20
3-10	-872	.67	7-13	449	.16
3-11	623	.75			

MAX DEFLECTION (span):  
 /999 IN MEM 11-12 (LIVE)  
 = -.12" D = -.07" T = -.12"

Joint Locations

1	0-0-0	8-25-4-0
2	4-2-0	9-0-0-0
3	5-0-0	10-4-2-0
4	8-4-0	11-8-4-0
5	13-10-6	12-13-4-0
6	15-0-3	13-16-7-7
7	19-4-12	14-25-4-0

Plating spec: ANSI/TPI - 1995  
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.  
 BEARING REQUIREMENTS shown are based ONLY on the truss material at each bearing.  
 PLATE VALUES PER ICHO RESEARCH REPORT #1607.  
 Drainage must be provided to avoid ponding.

1x4 continuous lateral WEB bracing (CIB), nailed w/2-10d, located for equal segments. CR 1x4 "T" brace nailed flat to edge of web w/ 8d at 8" o.c. CR a scab (same as web) nailed to face of web w/10d at 8" o.c. If 2 are required, attach "T" to both edges or scab to both faces. If 3 or more are req., use CIB. "T" or scab must be 90% of web length, and 2x6 if web is > 14'-0". CIB design is per building designer. Loaded for 10 PSF non-concurrent RLL. End verticals designed for axial loads only. End verticals that are extended above or below the truss profile (if any) may require additional design consideration (by others) for lateral forces due to wind or seismic loads on the building.

UPLIFT REACTION(S):  
 support 1 -633#  
 support 2 -471#  
 Hip/Rip: 0-1-13  
 This truss is designed using the UBC-97 Code.  
 Bldg Enclosed = Yes, Bldg Zone = No  
 (Hurricane/Ocean Line = No, Exp Category = C  
 Bldg Length = 99.00ft, Bldg Width = 50.00ft,  
 Mean roof height = 26.13ft, MH = 100  
 Classification = 3, Dead Load = 17.0 paf

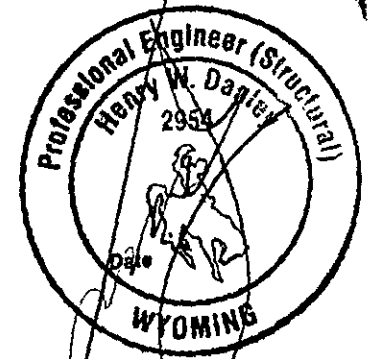
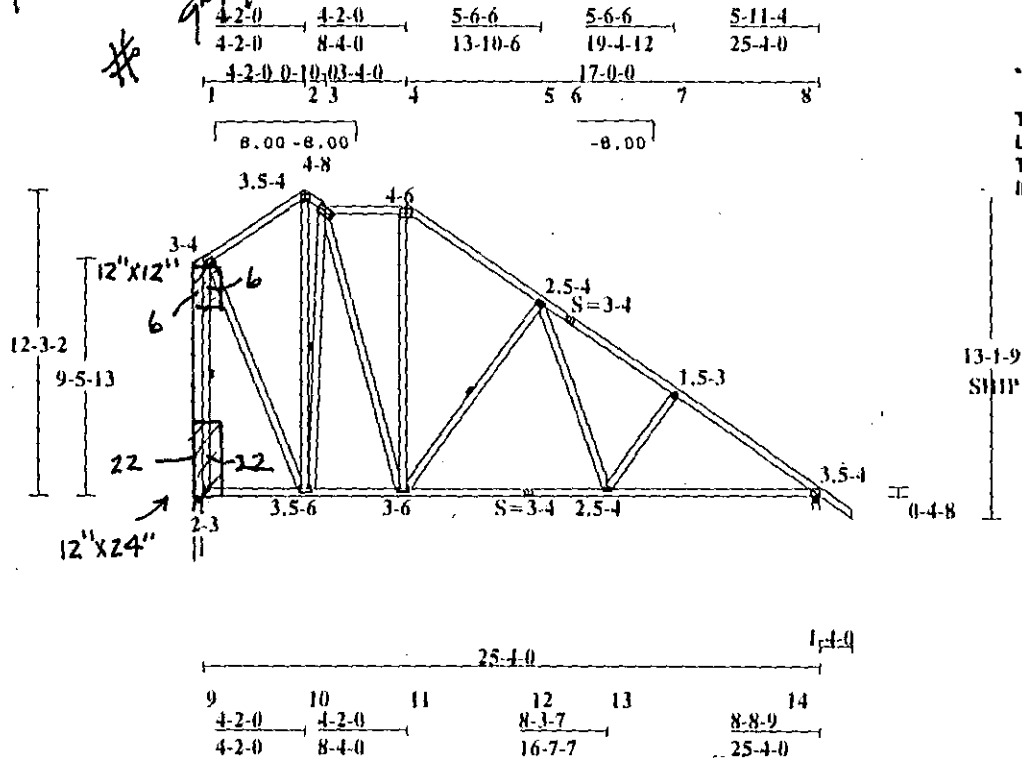
REPAIR-TRUSS MUST BE EXTENDED 3.5" ON THE LEFT END AS SHOWN.

- \* ADEQUATELY SUPPORT THE TRUSS UNTIL THE REPAIR IS COMPLETE.
  - \* LET IN A 2 X 4 # 2 DF-L VERTICAL.
  - \* APPLY 7/16", APA (MIN. SPAN RATING 24/16), EXPOSURE 1, PLYWOOD (OR OSB) TO ONE FACE WITH 8d NAILS AS SPECIFIED PER GUSSET. THE NAILS MUST BE EVENLY DISTRIBUTED THROUGHOUT.
  - \* APPLY ANY ADDITIONAL LATERAL BRACING IF SHOWN.
- THE NUMBER ASSOCIATED WITH A LINE POINTING TO THE GUSSET IS THE AMOUNT OF NAILS REQUIRED IN THE MEMBER PER GUSSET.

FILE: 1000 BEL VEDERE OF 99/1051

8.14.99 JPC

1 of 10



5/19/99  
 Scale: 1/8" = 1'  
 REF. CO99132040-039 5-12-99

TRUSSES, INC.  
 CASPER, WY.  
 Tp5.0 Version 09.30.98

**WARNING** Read all notes on this sheet and give a copy of it to the Erecting Contractor.  
 This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: "TRUSCOM MANUAL", by Truswal, "QUALITY CONTROL STANDARDS FOR METAL PLATE CONNECTED WOOD TRUSSES" - (QST-88), "HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" - (BIB-91) and "BIB-91 SUMMARY SHEET" by TPI. The Truss Plate Institute (TPI) is located at 583 D'Oroffrio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave, NW, Ste 200, Washington, DC 20036.

TBP:	102.0	WO:	HARMS102
Chk:	RP	Customer Name:	NORM HARMS
Degrn:	RP	#LC =	6
TC Live	30.0 paf	DirFaca	L=1.15 P=1.15
TC Dead	7.0 paf	Rep Mbr End	1.15
BC Live	.0 paf	O.C.Spacing	2- 0- 0
BC Dead	10.0 paf	Design Spec	UBC-97
TOTAL	47.0 paf	Defl Ratio:	1/240 RC: 1/240

PG	X-LOC	REACT	SIZE	REQ'D
1	0-1-12	1191	3.50"	1.50"
2	25-2-4	1376	3.50"	1.50"

TOP CHORD 2x4 FL #1  
 BOT CHORD 2x4 FL #1  
 WEB 2x4 FL #1  
 2x4 FL STUD 12-6, 6-13  
 13-7, 14-7

Plating spec : ANSI/TPI - 1995  
 THIS DESIGN IS THE COMPOSITE RESULT OF  
 MULTIPLE LOAD CASES.  
 BEARING REQUIREMENTS shown are based ONLY  
 on the truss material at each bearing.  
 Loaded for 10 PSF non-concurrent ECLL.

1x1 continuous lateral WEB bracing (CLB),  
 nailed w/2-10d, located for equal segments.  
 OR 1x4 "T" brace nailed flat to edge of web  
 w/ 8d at 8" o.c. OR a scab (same as web)  
 nailed to face of web w/10d at 8" o.c. If  
 2 are required, attach "T" to both edges or  
 scab to both faces. If 3 or more are req.,  
 use CLB. "T" or scab must be 90% of web  
 length, and 2x6 if web is > 14'-0". CLB  
 design is per building designer.  
 PLATE VALUES PER ICCO RESEARCH REPORT #1607.  
 End verticals designed for axial loads only.  
 End verticals that are extended above or  
 below the truss profile (if any) may require  
 additional design consideration (by others)  
 for lateral forces due to wind or seismic  
 loads on the building.

UPLIFT REACTION(S) :  
 support 1 -638#  
 support 2 -467#  
 This truss is designed using the  
 UBC-97 Code.  
 Bldg Enclosed = Yes, End Zone = No  
 Hurricane/Ocean Line = No, Exp Category = C  
 Bldg Length = 99.00ft, Bldg Width = 50.00ft,  
 Mean roof height = 26.69ft, MHF = 100  
 Classification = 3, Dead load = 17.0 psf

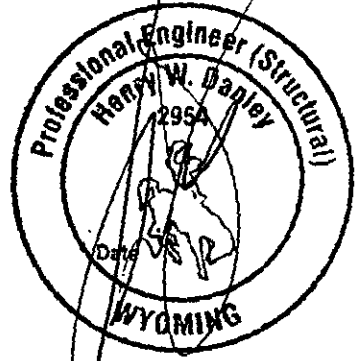
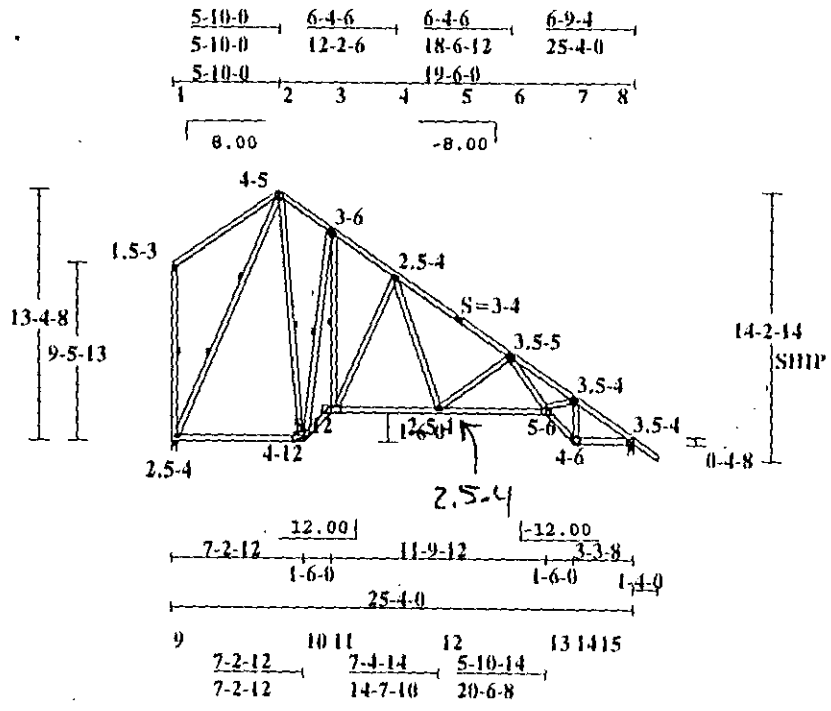
LC	FORCE	AXL	END	CSI
1	183	.00	.48	.48
2	-603	.00	.39	.40
3	-915	.06	.39	.45
4	-1525	.09	.39	.48
5	-1646	.05	.40	.45
6	-3175	.20	.37	.57
7	-1771	.03	.20	.23

WEB	FORCE	CSI	WEB	FORCE	CSI
1-9	249	.10	4-12	711	.31
2-9	-1010	.57	6-12	-848	.41
2-10	832	.49	6-13	1189	.40
3-10	-1331	.94	7-13	1264	.42
3-11	1354	.46	7-14	-1415	.29
4-11	-806	.74			

MAX DEFLECTION (span) :  
 /999 IN MEM 11-12 (LIVE)  
 = -.15" D= -.09" T= -.24"

Joint Locations

1	0-0-0	9	0-0-0
2	5-10-0	10	7-2-12
3	8-8-12	11	8-8-12
4	12-2-6	12	14-7-10
5	15-7-14	13	20-6-8
6	18-6-12	14	22-0-8
7	22-0-8	15	25-4-0
8	25-4-0		



5/19/99  
 Scale: 3/32" = 1'

REF. CO99132040-040 5-12-99

**TRUSSES, INC.**

CASPER, WY.

Truss 5.0 Version 09.30.98

**WARNING** Read all notes on this sheet and give a copy of it to the Erecting Contractor.  
 This design is for an individual building component. It has been based on specifications provided by the component manufacturer and does not conform to the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: "TRUSSMANUAL", by Trussal, "QUALITY CONTROL STANDARD FOR METAL PLATE CONNECTED WOOD TRUSSES" - (QST-MC), "HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" - (HIB-91) and "HIB-91 SUMMARY SHEET" by TPI. The Truss Plate Institute (TPI) is located at 583 D'Omotrio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave, NW, Ste 200, Washington, DC 20004.

TBP:	102.0	WO:	HARMS102
Chk:	RP	Customer Name:	NORM HARMS
Design:	RP	#LC =	6
TC Live	30.0 psf	DirFacs	L=1.15 P=1.15
TC Dead	7.0 psf	Rep Mbr Bnd	1.15
BC Live	.0 psf	O.C. Spacing	2- 0- 0
BC Dead	10.0 psf	Design Spec	UBC-97
TOTAL	47.0 psf	Defl Ratio:	1/240 TC: 1/240

NO	X-LOC	REACT	SIZE	REQ'D
1	0-1-12	1266	3.50"	1.50"
2	22-10-4	1081	3.50"	1.50"

NO	FORCE	AXL	END	CSI
1	-1539	.04	.55	.60
2	-980	.03	.55	.58
3	-701	.04	.15	.19
4	-762	.04	.25	.30
5	42	.00	.09	.09

NO	FORCE	AXL	END	CSI
1	1181	.19	.18	.37
2	1179	.19	.18	.37
3	1179	.19	.08	.27
4	596	.05	.23	.28
5	680	.01	.25	.26

NO	WEB FORCE	CSI	NO	WEB FORCE	CSI
2-8	253	.08	4-11	-948	.43
2-10	-592	.52	5-11	1143	.66
3-10	197	.03	5-12	-971	.80
4-10	-329	.40	6-12	-63	.04

Plating spec : ANSI/TPI - 1995  
**THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.**  
 BEARING REQUIREMENTS shown are based ONLY on the truss material at each bearing.  
**PLATE VALLES PER ICBO RESEARCH REPORT #1607.**  
 Drainage must be provided to avoid ponding.

1x4 continuous lateral WEB bracing (CIB), nailed w/2-10d, located for equal segments. OR 1x4 "T" brace nailed flat to edge of web w/ 8d at 8" o.c. OR a scab (same as web) nailed to face of web w/10d at 8" o.c. If 2 are required, attach "T" to both edges or scab to both faces. If 3 or more are req., use CIB. "T" or scab must be 90% of web length, and 2x6 if web is > 14'-0". CIB design is per building designer.  
 Loaded for 10 psf non-concentrated BCII.  
 End verticals designed for social loads only. End verticals that are extended above or below the truss profile (if any) may require additional design consideration (by others) for lateral forces due to wind or seismic loads on the building.

UNIFIED REACTION(S) :  
 support 1 -360#  
 support 2 -651#  
 HORIZONTAL REACTION(S) :  
 support 1 77#  
 support 2 77#  
 Hip-Trap : 0-1-13  
 This truss is designed using the UBC-97 Code.  
 Bldg Enclosed = Yes, End Zone = No  
 Hurricane/Ocean Line = No, Exp Category = C  
 Bldg Length = 99.00ft, Bldg Width = 50.00ft,  
 Mean roof height = 26.13ft, MPH = 100  
 Classification = 3, Dead load = 17.0 psf

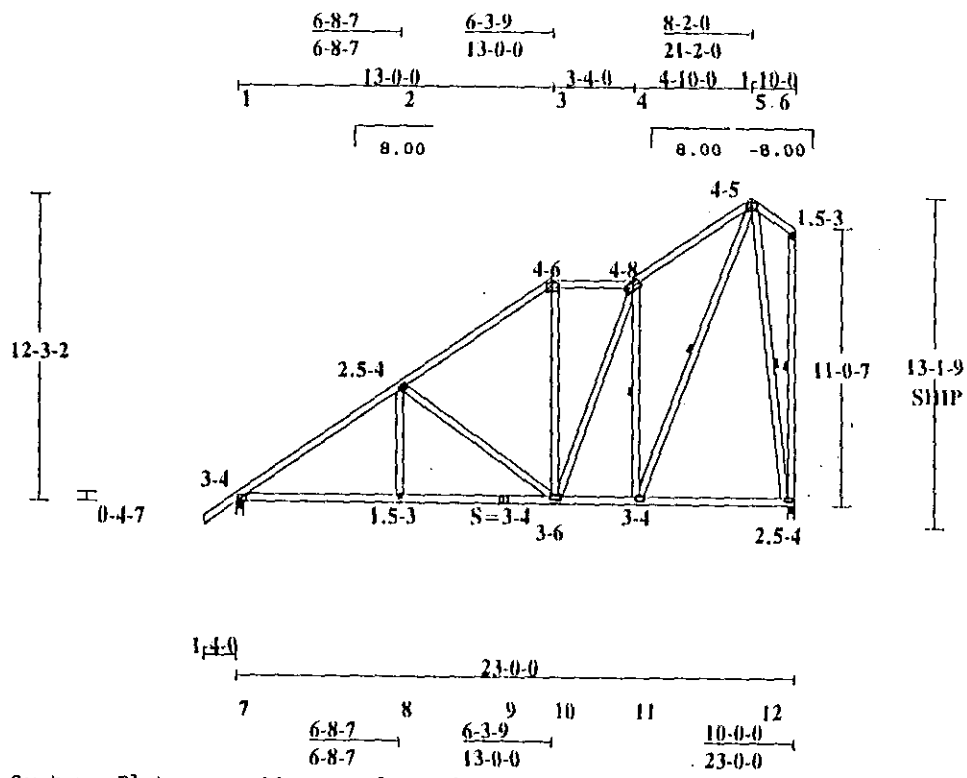
**REPAIR-TRUSS MUST BE EXTENDED 3.5" ON THE RIGHT END AS SHOWN.**

**SEE THE REPAIR ON PAGE 001 (T-36).**

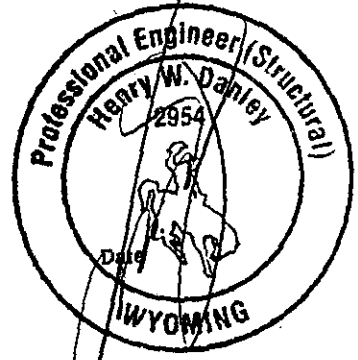
MAX DEFLECTION (span) :  
 /999 IN MEM 11-12 (LIVE)  
 = -.08" D= -.04" T= -.12"

==== Joint Locations =====

1	0-0-0	7	0-0-0
2	6-8-7	8	6-8-7
3	13-0-0	9	11-0-0
4	16-4-0	10	13-0-0
5	21-2-0	11	16-4-0
6	23-0-0	12	23-0-0



Trussal Systems Plates are 20 ga. unless shown by "18" (18 ga.) or "H" (16 ga.), positioned per Joint Report. Circled plates and false frame plates are positioned as shown above.



5/19/99

Scale: 1/8" = 1'

REF. CO99132040-042 5-12-99

**TRUSSES, INC.**  
 GASPERS, WY.  
 Tp5.0 Version 09.30.98

**WARNING** Read all notes on this sheet and give a copy of it to the Erecting Contractor.  
 This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: "TRUSCOM MANUAL", by Trussal, "QUALITY CONTROL STANDARD FOR METAL PLATE CONNECTED WOOD TRUSSES" - (QST-88), "HANDLING INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" - (HIB-91) and "HIB-91 SUMMARY SHEET" by TPI. The Truss Plate Institute (TPI) is located at 583 D'Onofrio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave, NW, Ste 200, Washington, DC 20036.

TBF:	91.3	WO:	HARMS102
Chk:	RP	Customer Name:	NORM HARMS
Dsgnr:	RP	#LC =	6
TC Live	30.0 psf	DurFacs	L=1.15 P=1.15
TC Dead	7.0 psf	Rep Mbr End	1.15
BC Live	.0 psf	O.C.Spacing	2- 0- 0
BC Dead	10.0 psf	Design Spec	UBC-97
TOTAL	47.0 psf	Defl Ratio:	1/240 TC: 1/240

ARG	X-LOC	REACT	SIZE	REQ'D
1	0-1-12	1266	3.50"	1.50"
2	22-10-4	1081	3.50"	1.50"

TC FORCE	AXL	END	CSI
1	-1539	.04	.55 .60
2	-980	.03	.55 .58
3	-701	.04	.15 .19
4	-762	.04	.25 .30
5	42	.00	.09 .09

TC FORCE	AXL	END	CSI
1	1181	.19	.18 .37
2	1179	.19	.18 .37
3	1179	.19	.08 .27
4	596	.05	.23 .28
5	680	.01	.25 .26

WEB FORCE	CSI	WEB FORCE	CSI
2-8	253 .08	4-11	-948 .43
2-10	-592 .52	5-11	1143 .66
3-10	197 .03	5-12	-971 .80
4-10	-329 .40	6-12	-63 .04

TOP CHORD 2x4 FL #1  
 BOT CHORD 2x4 FL #1  
 WEB 2x4 FL #1  
 2x4 FL STUD 8-2

Plating spec : ANSI/TPI - 1995  
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.  
 BEARING REQUIREMENTS shown are based ONLY on the truss material at each bearing.  
 PLATE VALLES PER ICHO RESEARCH REPORT #1607.  
 Drainage must be provided to avoid ponding.  
 This truss is designed using the UBC-97 Code.  
 Bldg Enclosed = Yes, End Zone = No  
 Hurricane/Ocean Line = No, Exp Category = C  
 Bldg Length = 99.00ft, Bldg Width = 50.00ft,  
 Mean roof height = 26.13ft, MFI = 100  
 Classification = 3, Dead Load = 17.0 psf

1x4 continuous lateral WEB bracing (CIB), nailed w/2-10d, located for equal segments. OR 1x4 "T" brace nailed flat to edge of web w/ 8d at 8" o.c. OR a scab (same as web) nailed to face of web w/10d at 8" o.c. If 2 are required, attach "T" to both edges or scab to both faces. If 3 or more are req., use CIB. "T" or scab must be 90% of web length, and 2x6 if web is > 14'-0". CIB design is per building designer.  
 Loaded for 10 PSF non-concurrent BCII.  
 End verticals designed for axial loads only. End verticals that are extended above or below the truss profile (if any) may require additional design consideration (by others) for lateral forces due to wind or seismic loads on the building.

This design based on chord bracing applied per the following schedule:  
 max o.c. from to  
 24'-0" 21'-2'-0" 20'-10'-14"  
 UPLIFT REACTION(S) :  
 support 1 -360#  
 support 2 -651#  
 HORIZONTAL REACTION(S) :  
 support 1 774#  
 support 2 774#  
 Hip-Drop : 0-1-13

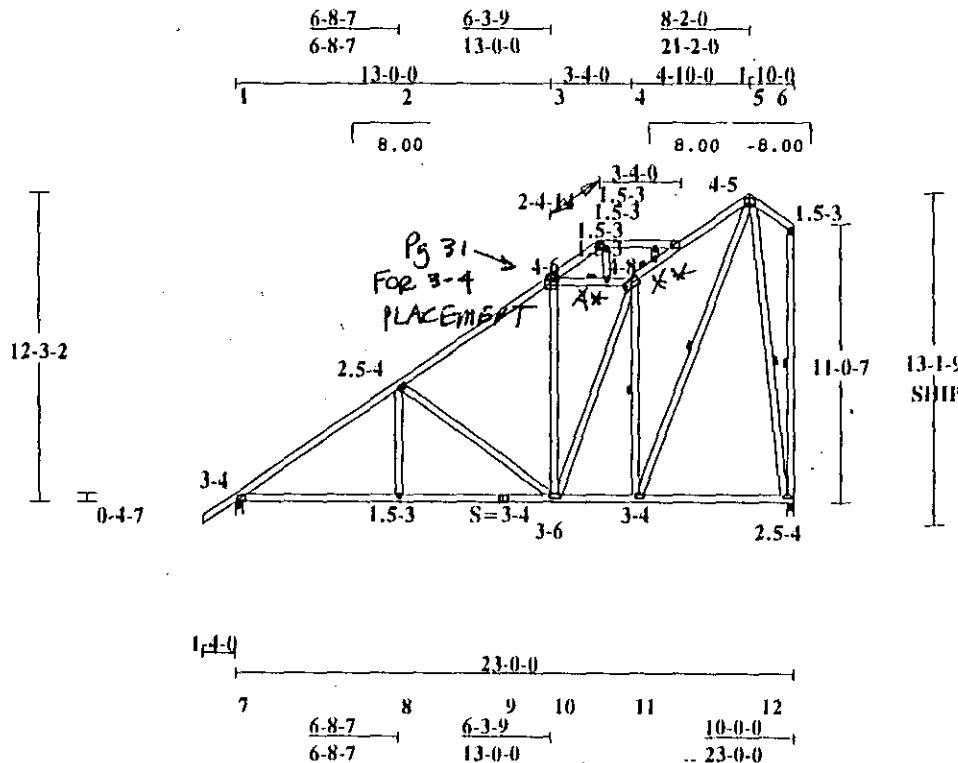
REPAIR-TRUSS MUST BE EXTENDED 3.5" ON THE RIGHT END AS SHOWN.

SEE THE REPAIR ON PAGE 001 (T-35).

MAX DEFLECTION (span) :  
 1/999 IN MEM 11-12 (LIVE)  
 S = -.08" D = -.04" T = -.12"

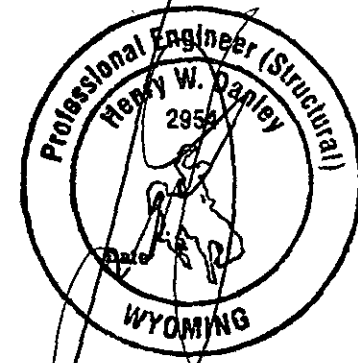
Joint Locations

1	0-0-0	7	0-0-0
2	6-8-7	8	6-8-7
3	13-0-0	9	11-0-0
4	16-4-0	10	13-0-0
5	21-2-0	11	16-4-0
6	23-0-0	12	23-0-0



TYPICAL PLATE : 3-4

Trussal Systems Plates are 20 ga. unless shown by "18" (18 ga.) or "H" (16 ga.), positioned per Joint Report. Circled plates and false frame plates are positioned as shown above.



5/19/99

Scale: 1/8" = 1'

REF. CO99132040-043 5-12-99



CASPER, WY.

Trp5.0 Version 09.30.98

**WARNING** Read all notes on this sheet and give a copy of it to the Erecting Contractor.

This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: "TRUSCOM MANUAL", by Trussal, "QUALITY CONTROL STANDARD FOR METAL PLATE CONNECTED WOOD TRUSSES" - (QST-88), "HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" - (HIB-91) and "HIB-91 SUMMARY SHEET" by TPI. The Truss Plate Institute (TPI) is located at 583 D'Oonofrio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave, NW, Ste 200, Washington, DC 20036.

TBF:	98.0	WO:	HARMS102
Chk:	RP	Customer Name:	NORM HARMS
Dagmr:	RP	#IC =	6
TC Live	30.0 psf	DurFacs	L=1.15 P=1.15
TC Dead	7.0 psf	Rep Mbr End	1.15
BC Live	.0 psf	O.C.Spacing	2- 0- 0
BC Dead	10.0 psf	Design Spec	UBC-97
TOTAL	47.0 psf	Defl Ratio:	1/240 TC: 1/240

ARG	X-LCC	REACT	SIZE	REQ'D
1	0-1-12	2474	3.50"	1.50"
2	22-10-4	3788	3.50"	2.02"

TC FORCE	AXL	END	CSI
1	41	.01	.15
2	-1051	.01	.23
3	-1381	.00	.15
4	-2761	.00	.27
5	-6101	.17	.39

TC FORCE	AXL	END	CSI
1	791	.01	.25
2	1280	.06	.12
3	1453	.07	.02
4	1453	.07	.05
5	2922	.13	.17
6	4993	.22	.17

TOP CHORD 2x4 FL #1  
 BOT CHORD 2x6 FL 1800F-1.6E  
 WEB 2x4 FL #1  
 2x4 FL STUD 11-5, 5-12

PLATE VALLES PER ICSO RESEARCH REPORT #1607.  
 \*\*\*\*\*  
 Nail pattern shown is for H.F. loads only.  
 Concentrated loads MBT be distributed to each ply equally. On multi-ply with hangers, use 3" nails min. into the carrying member.  
 For more than 2 ply, add nail cluster from additional ply(s) and/or bolts if shown, or use any other approved detail (by others).  
 \*\*\*\*\*  
 2-PLY! Nail w/10d BOX, staggered (NDS-91 Sect. 12) in: TC- 2/ft BC- 2/ft WEBS- 2/ft

Plating spec : ANSI/TPI - 1995  
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.  
 BEARING REQUIREMENTS shown are based ONLY on the truss material at each bearing.  
 Load for 10 PSF non-concurrent BC/L.  
 Drainage must be provided to avoid ponding.  
 Permanent bracing is required (by others) to prevent rotation/toppling. See HIB-91 and ANSI/TPI 1-1995; 10.3.4.5 and 10.3.4.6.  
 End verticals designed for axial loads only.  
 End verticals that are extended above or below the truss profile (if any) may require additional design consideration (by others) for lateral forces due to wind or seismic loads on the building.

UPLIFT REACTION(S) :  
 support 1 -770#  
 support 2 -1179#

This truss is designed using the UBC-97 Code.  
 Bldg Enclosed = Yes, End Zone = No  
 Hurricane/Ocean Line = No, Exp Category = C  
 Bldg Length = 99.00ft, Bldg Width = 50.00ft,  
 Mean roof height = 26.13ft, MHI = 100  
 Classification = 3, Dead Load = 17.0 psf

-----LOAD CASE #1 DESIGN LOADS-----

Dir	L.PLF	L.LoC	R.PLF	R.LoC	IL/TL
TC Vert	74.0	0-0-0	74.0	23-0-0	.81
BC Vert	295.0	0-0-0	295.0	4-0-0	.60
BC Vert	20.0	4-0-0	20.0	23-0-0	.00

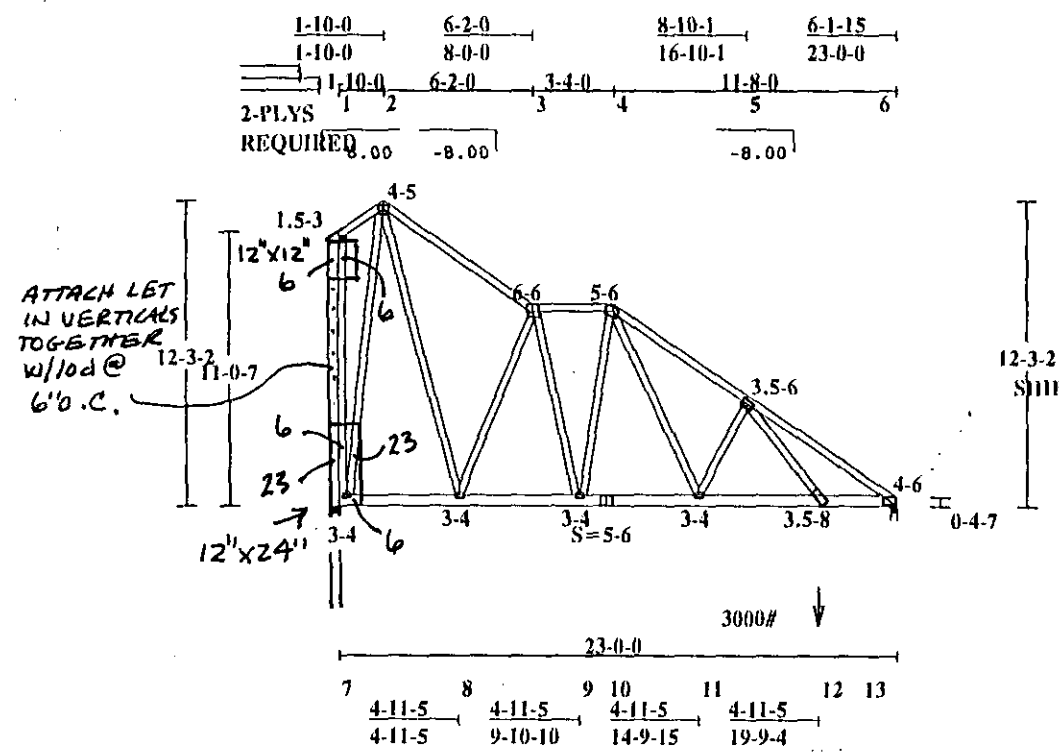
...Type... lbs X.LoC IL/TL  
 BC Vert 3000.0 19-9-4 .64

WEB FORCE	CSI	WEB FORCE	CSI
1-7	-54 .01	4-9	-407 .05
2-7	-1778 .46	4-11	1750 .14
2-8	2028 .17	5-11	-1521 .16
3-8	-1391 .19	5-12	3457 .58
3-9	405 .03		

MAX DEFLECTION (span) :  
 L/999 IN MEM 11-12 (LIVE)  
 Δ = -.07" D = -.04" T = -.11"

Joint Locations

1	0-0-0	8	4-11-5
2	1-10-0	9	9-10-10
3	8-0-0	10	11-0-0
4	11-4-0	11	14-9-15
5	16-10-1	12	19-9-4
6	23-0-0	13	23-0-0
7	0-0-0		

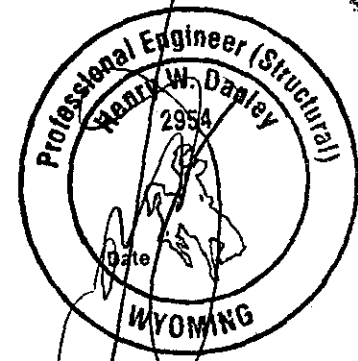


Trussal Systems Plates are 20 ga. unless shown by "18" (18 ga.) or "H" (16 ga.), positioned per Joint Report. Circled plates and false frame plates are positioned as shown above.

REPAIR-TRUSS MUST BE EXTENDED 3.5' ON THE LEFT END AS SHOWN.

- \* ADEQUATELY SUPPORT THE TRUSS UNTIL THE REPAIR IS COMPLETE.
- \* LET IN A 2 PLY 2 X 4 # 2 DF-L VERTICAL.
- \* APPLY 7/16" APA (MIN. SPAN RATING 24/16), EXPOSURE 1, PLYWOOD (OR OSB) TO EACH FACE WITH 8d NAILS AS SPECIFIED PER GUSSET. THE NAILS MUST BE EVENLY DISTRIBUTED THROUGHOUT.
- \* APPLY ANY ADDITIONAL LATERAL BRACING IF SHOWN.

THE NUMBER ASSOCIATED WITH A LINE POINTING TO THE GUSSET IS THE AMOUNT OF NAILS REQUIRED IN THE MEMBER PER GUSSET.



5/19/99  
 Scale: 1/8" = 1'  
 S-12-99

REF. CO99132040-004

**TRUSSES, INC.**

CASPER, WY.

Trp5.0 Version 09.30.98

**WARNING** Read all notes on this sheet and give a copy of it to the Erecting Contractor.  
 This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: 'TRUSCOM MANUAL', by Trussal, 'QUALITY CONTROL STANDARD FOR METAL PLATE CONNECTED WOOD TRUSSES' - (QST-88), 'HANDLING INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES' - (HIB-91) and 'HIB-91 SUMMARY SHEET' by TPI. The Truss Plate Institute (TPI) is located at 583 D'Onofrio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave, NW, Ste 200, Washington, DC 20036.

TBF:	98.0	WO:	HARMS102
Chk:	RP	Customer Name:	NORM HARMS
Dgnr:	RP	#LC =	6
TC Live	30.0 psf	DarFace	L=1.15 P=1.15
TC Dead	7.0 psf	Rep Mbr Bnd	1.00
BC Live	.0 psf	O.C.Spacing	2-0-0
BC Dead	10.0 psf	Design Spec	UBC-97
TOTAL	47.0 psf	Defl Ratio:	1/240 TC: 1/240

RG	X-LOC	REACT	SIZE	REQ'D
1	0-1-12	1833	3.50"	1.96"
2	19-6-4	1808	3.50"	1.93"

TC FORCE	AXL	BND	CSI
1	-1653	.03	.64 .66
2	-2362	.06	.64 .69
3	-2079	.04	.58 .62
4	-2435	.06	.28 .34
5	-2562	.07	.15 .22

TOP CHORD 2x4 FL #1  
 BOT CHORD 2x6 FL 1800F-1.6E  
 WEB 2x4 FL STD

Lumber shear allowables are per NDS-97.  
 PLATE VALUES PER ICBO RESEARCH REPORT #1607.  
 Drainage must be provided to avoid ponding.  
 End verticals designed for axial loads only.  
 End verticals that are extended above or below the truss profile (if any) may require additional design consideration (by others) for lateral forces due to wind or seismic loads on the building.

Plating spec : ANSI/TPI - 1995  
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.  
 BEARING REQUIREMENTS shown are based ONLY on the truss material at each bearing.  
 Loaded for 10 PSF non-occupant KILL.  
 Remanent bracing is required (by others) to prevent rotation/toppling. See HIB-91 and ANSI/TPI 1-1995; 10.3.4.5 and 10.3.4.6.

UPLIFT REACTION(S) :  
 support 1 -514#  
 support 2 -507#

Hip-Drp : 0-1-13  
 This truss is designed using the UBC-97 Code.  
 Bldg Enclosed = Yes, End Zone = No  
 Hurricane/Ocean Line = No , Exp Category = C  
 Bldg Length = 99.00ft, Bldg Width = 50.00ft,  
 Mean roof height = 22.18ft, MHF = 100  
 Classification = 3, Dead Load = 17.0 psf

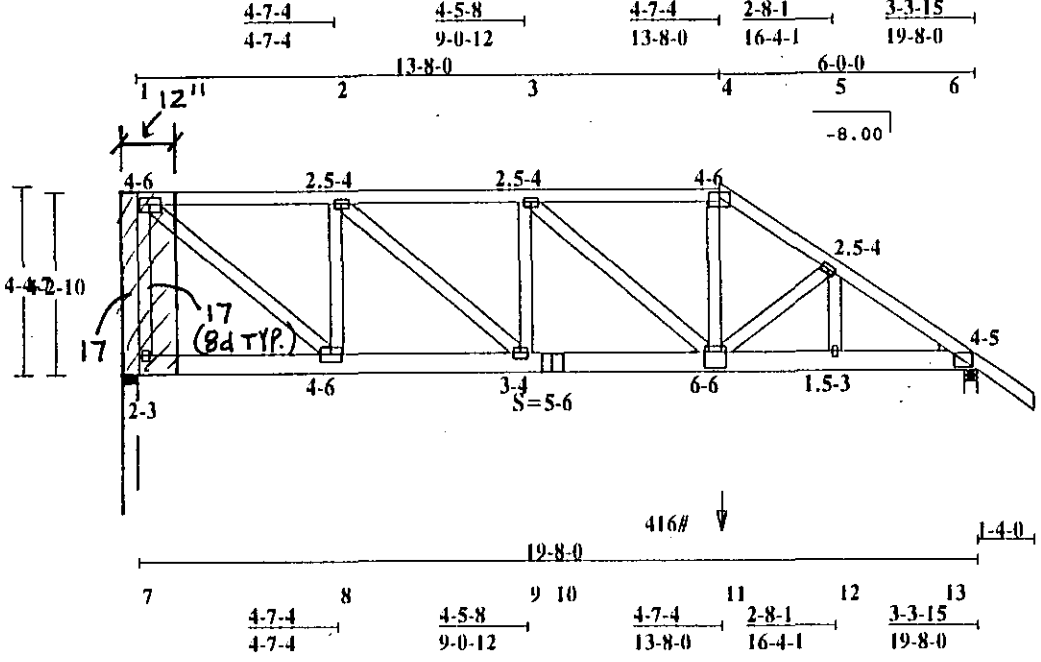
TC FORCE	AXL	BND	CSI
1	281	.00	.07 .07
2	1729	.15	.06 .21
3	2386	.21	.03 .24
4	2386	.21	.06 .28
5	2075	.19	.07 .26
6	2074	.19	.07 .26

WEB FORCE	CSI	WEB FORCE	CSI
1-7	-1749 .53	3-11 -409 .24	
1-8	2208 .74	4-11 904 .30	
2-8	-1287 .39	5-11 197 .05	
2-9	854 .29	5-12 40 .01	
3-9	-406 .12		

MAX DEFLECTION (span) :  
 √999 IN MEM 9-10 (LIVE)  
 = -.07" D= -.04" T= -.12"

Joint Locations

1	0-0-0	8	4-7-4
2	4-7-4	9	9-0-12
3	9-0-12	10	9-8-0
4	13-8-0	11	13-8-0
5	16-4-1	12	16-4-1
6	19-8-0	13	19-8-0
7	0-0-0		



LOAD CASE #1 DESIGN LOADS

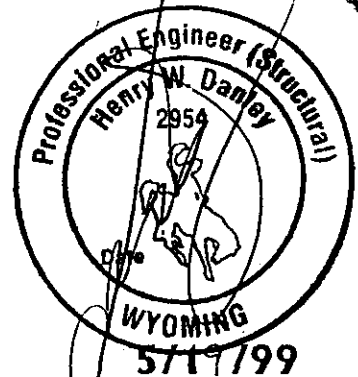
Dir	L.Plf	L.Loc	R.Plf	R.Loc	LL/TL
TC Vert	142.6	0-0-0	142.6	13-8-0	.81
TC Vert	74.0	13-8-0	74.0	19-8-0	.81
BC Vert	38.5	0-0-0	38.5	13-8-0	.00
BC Vert	20.0	13-8-0	20.0	19-8-0	.00

Type	Lbs	X.Loc	LL/TL
BC Vert	265.8	13-8-0	1.00
BC Vert	150.6	13-8-0	.00

REPAIR-TRUSS MUST BE EXTENDED 3.5" ON THE LEFT END AS SHOWN.

- \* ADEQUATELY SUPPORT THE TRUSS UNTIL THE REPAIR IS COMPLETE.
  - \* LET IN A 2 PLY 2 X 4 # 2 DF-L VERTICAL.
  - \* APPLY 7/16", APA (MIN. SPAN RATING 24/16), EXPOSURE 1, PLYWOOD (OR OSB) TO EACH FACE WITH 8d NAILS AS SPECIFIED PER GUSSET. THE NAILS MUST BE EVENLY DISTRIBUTED THROUGHOUT.
  - \* APPLY ANY ADDITIONAL LATERAL BRACING IF SHOWN.
- THE NUMBER ASSOCIATED WITH A LINE POINTING TO THE GUSSET IS THE AMOUNT OF NAILS REQUIRED IN THE MEMBER PER GUSSET.



Scale: 7/32" = 1'

Trussal Systems Plates are 20 ga. unless shown by "18" (18 ga.) or "H" (16 ga.), positioned per Joint Report. Circled plates and false frame plates are positioned as shown above.

REF. CO99132040-047 5-12-99

**WARNING** Read all notes on this sheet and give a copy of it to the Erecting Contractor.  
 This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: 'TRUSCOM MANUAL', by Trussal, 'QUALITY CONTROL STANDARD FOR METAL PLATE CONNECTED WOOD TRUSSES' - (QST-88), 'HANDLING INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES' - (HIB-91) and 'HIB-91 SUMMARY SHEET' by TPI. The Truss Plate Institute (TPI) is located at 583 D'Onofrio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave. NW, Ste 200, Washington, DC 20036.

TBF:	62.7	WO:	HARMS102
Chk:	RP	Customer Name:	NORM HARMS
Dagnr:	RP	#LC =	6
TC Live	30.0 psf	DurFace	L=1.15 P=1.25
TC Dead	7.0 psf	Rep Mbr Bnd	1.00
BC Live	.0 psf	O.C.Spacing	2- 0- 0
BC Dead	10.0 psf	Design Spec	UBC-97
TOTAL	47.0 psf	Defl Ratio:	I/240 TC: I/240

ARG	X-LOC	REACT	SIZE	REQ'D
1	0-1-12	924	3.50"	1.50"
2	19-6-4	924	3.50"	1.50"

TOP CHORD	2x4	FL #1
BOT CHORD	2x4	FL #1
WEB	2x4	FL STUD
	2x4	FL #1

AXL	END	CSI
1	-1311	.06 .17 .23
2	-1063	.05 .15 .20
3	-834	.01 .56 .56
4	-740	.01 .56 .56

AXL	END	CSI
1	1033	.08 .28 .35
2	764	.06 .28 .34
3	764	.06 .22 .28
4	377	.00 .22 .22

WEB FORCE	CSI	WEB FORCE	CSI
2-7	318 .11	4-9	-580 .30
3-7	249 .08	5-9	1017 .43
4-7	56 .07	5-10	-875 .45

Plating spec : ANSI/TPI - 1995  
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.  
 BEARING REQUIREMENTS shown are based ONLY on the truss material at each bearing.  
 Loaded for 10 psf non-concurrent ECLL.  
 End verticals designed for axial loads only.  
 End verticals that are extended above or below the truss profile (if any) may require additional design consideration (by others) for lateral forces due to wind or seismic loads on the building.

UPLIFT REACTION(S) :  
 support 1 -343#  
 support 2 -451#  
 HORIZONTAL REACTION(S) :  
 support 1 377#  
 support 2 377#  
 This truss is designed using the UBC-97 Code.  
 Bldy Enclosed = Yes, End Zone = No  
 Hurricane/Ocean Ldr = No, Exp Category = C  
 Bldy Length = 99.00ft, Bldy Width = 50.00ft,  
 Mean roof height = 22.85ft, MEH = 100  
 Classification = 3, Dead Load = 17.0 psf

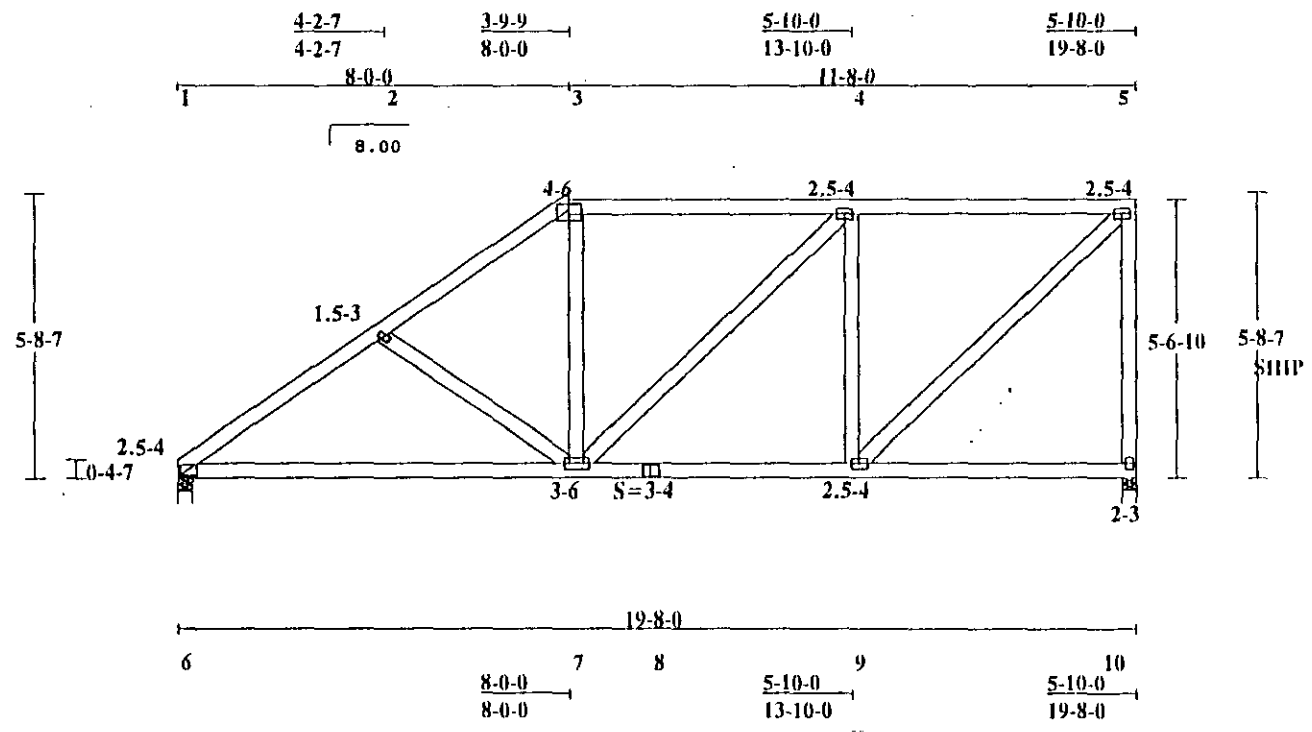
REPAIR-TRUSS MUST BE EXTENDED 3.5" ON THE RIGHT END AS SHOWN.

SEE THE REPAIR ON PAGE 006 (T-21).

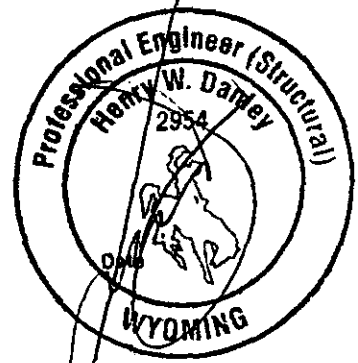
MAX DEFLECTION (span) :  
 /999 IN MEM 6-7 (LIVE)  
 = -.08" D= -.04" T= -.12"

Joint Locations

Joint	X	Y	Z
1	0-0-0	6	0-0-0
2	4-2-7	7	8-0-0
3	8-0-0	8	9-8-0
4	13-10-0	9	13-10-0
5	19-8-0	10	19-8-0



Trussal Systems Plates are 20 ga. unless shown by "18" (18 ga.) or "H" (16 ga.), positioned per Joint Report. Circled plates and false frame plates are positioned as shown above.



5/19/99  
 Scale: 1/4" = 1'

REF. CO99132040-048 5-12-99

**TRUSSES, INC.**  
 CASPER, WY.  
 Tp5.0 Version 09.30.98

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TBF:	52.0	WO:	HARMS102
Chk:	RP	Customer Name:	NORM HARMS
Dgnr:	RP	#LC =	6
TC Live	30.0 psf	DurFacs	L=1.15 P=1.15
TC Dead	7.0 psf	Rep Mbr Bnd	1.15
BC Live	.0 psf	O.C.Spacing	2- 0- 0
BC Dead	10.0 psf	Design Spec	UBC-97
TOTAL	47.0 psf	Defl Ratio:	I <sub>v</sub> /240 TC: I <sub>v</sub> /240

TRG	X-LOC	REACT	SIZE	REQ'D	TOP CHORD	2x4	FL	#1
1	0-1-12	924	3.50"	1.50"	BOT CHORD	2x4	FL	#1
2	19-6-4	924	3.50"	1.50"	WEB	2x4	FL	STUD
						2x4	FL	#1

TC	FORCE	AXL	END	CSI
1	-1311	.06	.17	.23
2	-1063	.05	.15	.20
3	-834	.01	.56	.56
4	-740	.01	.56	.56

BC	FORCE	AXL	END	CSI
1	1033	.08	.28	.35
2	764	.06	.28	.34
3	764	.06	.22	.28
4	377	.00	.22	.22

WEB	FORCE	CSI	WEB	FORCE	CSI
2-7	318	.11	4-9	-580	.30
3-7	249	.08	5-9	1017	.43
4-7	96	.07	5-10	-875	.45

PLATE VALUES PER ICEO RESEARCH REPORT #1607.  
 Drainage must be provided to avoid ponding.  
 Hip-Drop : 0-1-13

Plating spec : ANSI/TPI - 1995  
 THIS DESIGN IS THE COMPOSITE RESULT OF  
 MULTIPLE LOAD CASES.  
 BEARING REQUIREMENTS shown are based ONLY  
 on the truss material at each bearing.  
 Loaded for 10 PSF non-concurrent BOLL.  
 End verticals designed for axial loads only.  
 End verticals that are extended above or  
 below the truss profile (if any) may require  
 additional design consideration (by others)  
 for lateral forces due to wind or seismic  
 loads on the building.

UPLIFT REACTION(S) :  
 support 1 -343#  
 support 2 -451#  
 HORIZONTAL REACTION(S) :  
 support 1 377#  
 support 2 377#  
 This truss is designed using the  
 UBC-97 Code.  
 Bldg Enclosed = Yes, End Zone = No  
 Hurricane/Ocean Lins = No, Exp Category = C  
 Bldg Length = 99.00ft, Bldg Width = 50.00ft,  
 Mean roof height = 23.52ft, MHF = 100  
 Classification = 3, Dead Load = 17.0 psf

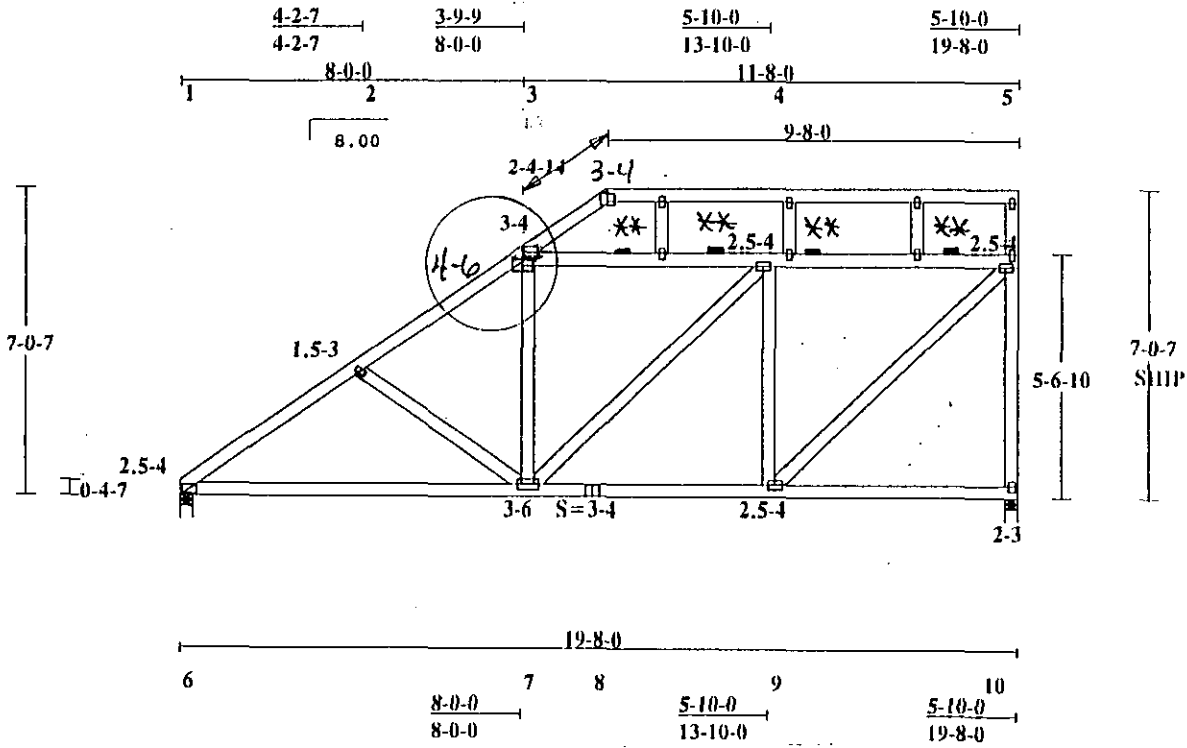
4X2 CONTINUOUS LATERAL COLUMN BRACING  
 ATTACHED WITH TWO (2) 10d NAILS TO EACH  
 MEMBER WHERE INDICATED BY (\*\*). BRACE  
 MATERIAL PER BUILDING DESIGNER.

REPAIR-TRUSS MUST BE EXTENDED  
 3.5" ON THE RIGHT END AS SHOWN.  
 USE THE SAME REPAIR ON PAGE 005 (T32)

MAX DEFLECTION (span) :  
 /999 IN MEM 6-7 (LIVE)  
 F = -.08" D = -.04" T = -.12"

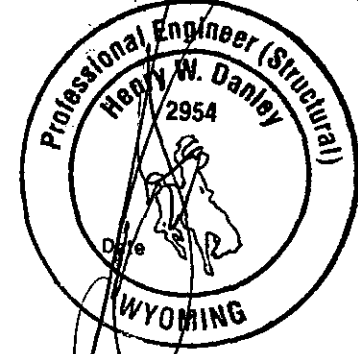
Joint Locations

1	0-0-0	6	0-0-0
2	4-2-7	7	8-0-0
3	8-0-0	8	9-8-0
4	13-10-0	9	13-10-0
5	19-8-0	10	19-8-0



TYPICAL PLATE : 1.5-3

Truswal Systems Plates are 20 ga. unless shown by "18" (18 ga.) or "H" (16 ga.), positioned per Joint Report. Circled plates and false frame plates are positioned as shown above.



5/19/99  
 Scale: 7/32" = 1'

REF. CO99132040-049 5-12-99

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TBF:	66.0	WO:	HARMS102
Chk:	RP	Customer Name:	NORM HARMS
Degrn:	RP #LC = 6	DurFacs	L=1.15 P=1.15
TC Live	30.0 psf	Rep Mbr Bnd	1.15
TC Dead	7.0 psf	O.C.Spacing	2- 0- 0
BC Live	.0 psf	Design Spec	UBC-97
BC Dead	10.0 psf	Defl Ratio:	L/240 TC: L/240
<b>TOTAL</b>	<b>47.0 psf</b>		



ARG	X-LOC	REACT	SIZE	REQ'D
1	0-1-12	924	3.50"	1.50"
2	19-6-4	924	3.50"	1.50"

TC FORCE	AXL	END	CSI
1	-1311	.06	.17 .23
2	-1063	.05	.15 .20
3	-834	.01	.56 .56
4	-740	.01	.56 .56

TC FORCE	AXL	END	CSI
1	1033	.08	.28 .35
2	764	.06	.28 .34
3	764	.06	.22 .28
4	377	.00	.22 .22

WEB FORCE	CSI	WEB FORCE	CSI
2-7	318 .11	4-9	-580 .30
3-7	249 .08	5-9	1017 .43
4-7	96 .07	5-10	-875 .45

TOP CHORD 2x4 FL #1  
 BOT CHORD 2x4 FL #1  
 WEB 2x4 FL STUD  
 2x4 FL #1 7-4, 9-5  
 PLATE VALLES PER ICBO RESEARCH REPORT #1607.  
 Drainage must be provided to avoid ponding.  
 Hip-Drop : 0-1-13

Plating spec : ANSI/TPI - 1995  
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.  
 BEARING REQUIREMENTS shown are based ONLY on the truss material at each bearing.  
 Loaded for 10 EBF non-concurrent BCIL.  
 End verticals designed for axial loads only.  
 End verticals that are extended above or below the truss profile (if any) may require additional design consideration (by others) for lateral forces due to wind or seismic loads on the building.

UNLIFT REACTION(S) :  
 support 1 -343#  
 support 2 -451#  
 HORIZONTAL REACTION(S) :  
 support 1 377#  
 support 2 377#  
 This truss is designed using the UBC-97 Code.  
 Bldg Enclosed = Yes, Bld Zone = No  
 Hurricane/Ocean Line = No, Exp Category = C  
 Bldg Length = 99.00ft, Bldg Width = 50.00ft,  
 Mean roof height = 24.19ft, MH = 100  
 Classification = 3, Dead Load = 17.0 psf

4X2 CONTINUOUS LATERAL COLUMN BRACING ATTACHED WITH TWO (2) 10d NAILS TO EACH MEMBER WHERE INDICATED BY (XX). BRACE MATERIAL PER BUILDING DESIGNER.

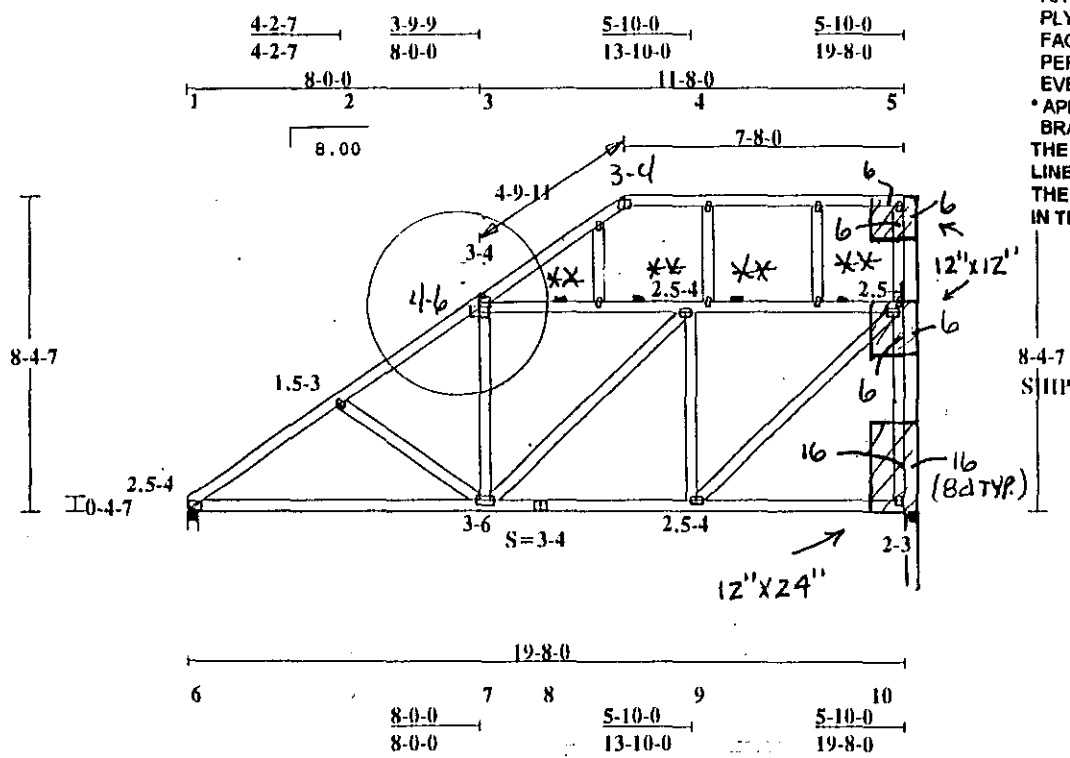
REPAIR-TRUSS MUST BE EXTENDED 3.5" ON THE RIGHT END AS SHOWN.

- \* ADEQUATELY SUPPORT THE TRUSS UNTIL THE REPAIR IS COMPLETE.
  - \* LET IN A 2 X 4 # 2 DF-L VERTICAL.
  - \* APPLY 7/16" APA (MIN. SPAN RATING 24/16), EXPOSURE 1, PLYWOOD (OR OSB) TO ONE FACE WITH 8d NAILS AS SPECIFIED PER GUSSET. THE NAILS MUST BE EVENLY DISTRIBUTED THROUGHOUT.
  - \* APPLY ANY ADDITIONAL LATERAL BRACING IF SHOWN.
- THE NUMBER ASSOCIATED WITH A LINE POINTING TO THE GUSSET IS THE AMOUNT OF NAILS REQUIRED IN THE MEMBER PER GUSSET.

MAX DEFLECTION (span) :  
 /999 IN MEM 6-7 (LIVE)  
 F = -.08" D = -.04" T = -.12"

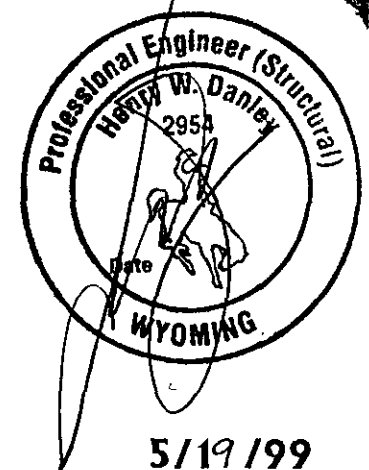
Joint Locations

1	0-0-0	6	0-0-0
2	4-2-7	7	8-0-0
3	8-0-0	8	9-8-0
4	13-10-0	9	13-10-0
5	19-8-0	10	19-8-0



TYPICAL PLATE : 1.5-3

Trussal Systems Plates are 20 ga. unless shown by "18" (18 ga.) or "H" (16 ga.), positioned per Joint Report. Circled plates and false frame plates are positioned as shown above.



5/19/99

Scale: 3/16" = 1'

REF. CO99132040-050 5-12-99

**TRUSSES, INC.**  
 CASPER, WY.  
 Tp5.0 Version 09.30.98

**WARNING** Read all notes on this sheet and give a copy of it to the Erecting Contractor.  
 This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: "TRUSCOM MANUAL", by Trussal, "QUALITY CONTROL STANDARD FOR METAL PLATE CONNECTED WOOD TRUSSES" - (QST-88), "HANDLING INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" - (HIB-91) and "HIB-91 SUMMARY SHEET" by TPI. The Truss Plate Institute (TPI) is located at 583 D'Onofrio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave, NW, Ste 200, Washington, DC 20006.

TBF:	68.7	WO:	HARMS102
Chk:	RP	Customer Name:	NORM HARMS
Degrn:	RP	#LC =	6
TC Live	30.0 psf	DurFacs	L=1.15 P=1.15
TC Dead	7.0 psf	Rep Mbr Bnd	1.15
BC Live	.0 psf	O.C.Spacing	2- 0- 0
BC Dead	10.0 psf	Design Spec	UBC-97
TOTAL	47.0 psf	Defl Ratio:	1/240 TC: 1/260

PG	X-LOC	REACT	SIZE	REQ'D	TOP CHORD	2x4	FL #1
1	0-1-12	924	3.50"	1.50"	BOT CHORD	2x4	FL #1
2	19-6-4	924	3.50"	1.50"	WEB	2x4	FL #1

TC FORCE	AXL	END	CSI	
1	65	.00	.24	.24
2	-868	.02	.55	.57
3	-640	.02	.27	.29
4	-796	.03	.20	.23
5	107	.01	.20	.22

BC FORCE	AXL	END	CSI	
1	519	.01	.45	.46
2	576	.05	.45	.50
3	576	.05	.20	.25
4	-691	.05	.18	.23

WEB FORCE	CSI	WEB FORCE	CSI
1-7	-21 .01	4-10	-190 .16
2-7	-849 .70	5-10	305 .07
2-8	994 .72	5-11	-952 .63
3-8	-814 .66	6-11	141 .03
4-8	176 .02		

Plating spec : ANSI/TPI - 1995  
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.  
 BEARING REQUIREMENTS shown are based ONLY on the truss material at each bearing.  
 Loaded for 10 PSF non-concurrent BCLL.  
 End verticals designed for axial loads only. End verticals that are extended above or below the truss profile (if any) may require additional design consideration (by others) for lateral forces due to wind or seismic loads on the building.

1x4 continuous lateral WEB bracing (CLB), nailed w/2-10d, located for equal segments. OR 1x4 "T" brace nailed flat to edge of web w/ 8d at 8" o.c. OR a scab (same as web) nailed to face of web w/10d at 8" o.c. If 2 are required, attach "T" to both edges or scab to both faces. If 3 or more are req., use CLB. "T" or scab must be 90% of web length, and 2x6 if web is > 14'-0". CLB design is per building designer.  
 HATE VALUES PER ICBO RESEARCH REPORT #1607. Drainage must be provided to avoid ponding.

UPLIFT REACTION(S) :  
 support 1 -603#  
 support 2 -189#  
 Hip-Data : 0-1-13  
 This truss is designed using the UBC-97 Code.  
 Bldg Enclosed = Yes, End Zone = No  
 Hurricane/Ocean Tide = No, Exp Category = C  
 Bldg Length = 99.00ft, Bldg Width = 50.00ft,  
 Mean roof height = 26.13ft, MEH = 100  
 Classification = 3, Dead Load = 17.0 psf

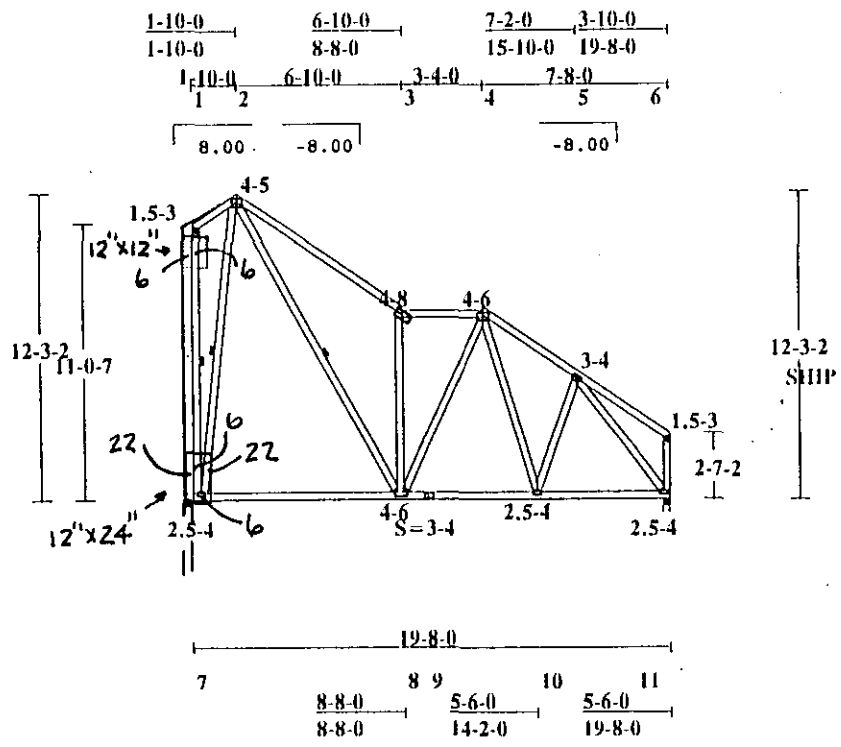
**REPAIR-TRUSS MUST BE EXTENDED 3.5" ON THE LEFT END, AS SHOWN.**

- \* ADEQUATELY SUPPORT THE TRUSS UNTIL THE REPAIR IS COMPLETE.
  - \* LET IN A 2 X 4 # 2 DF-L VERTICAL.
  - \* APPLY 7/16", APA (MIN. SPAN RATING 24/16), EXPOSURE 1, PLYWOOD (OR OSB) TO ONE FACE WITH 8d NAILS AS SPECIFIED PER GUSSET. THE NAILS MUST BE EVENLY DISTRIBUTED THROUGHOUT.
  - \* APPLY ANY ADDITIONAL LATERAL BRACING IF SHOWN.
- THE NUMBER ASSOCIATED WITH A LINE POINTING TO THE GUSSET IS THE AMOUNT OF NAILS REQUIRED IN THE MEMBER PER GUSSET.

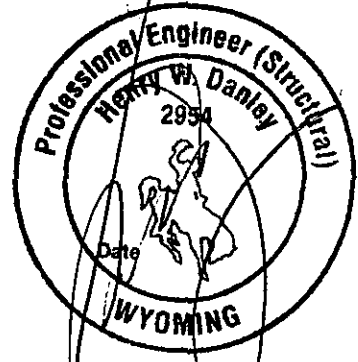
MAX DEFLECTION (span) :  
 L/999 IN MEM 7-8 (LIVE)  
 Δ = -.19" D = -.11" T = -.30"

Joint Locations

1	0-0-0	7	0-0-0
2	1-10-0	8	8-8-0
3	8-8-0	9	9-8-0
4	12-0-0	10	14-2-0
5	15-10-0	11	19-8-0
6	19-8-0		



Truswal Systems Plates are 20 ga. unless shown by "18" (18 ga.) or "11" (16 ga.), positioned per Joint Report. Circled plates and false frame plates are positioned as shown above.



5/19/99

Scale: 1/8" = 1'

REF. CO99132040-053 5-12-99

**TRUSSES, INC.**  
 CASPER, WY.  
 Tp5.0 Version 09.30.98

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TBF:	82.0	WO:	HARMS102
Chk:	RP	Customer Name:	NORM HARMS
Degrn:	RP #I.C = 6		
TC Live	30.0 psf	DurFace	L=1.15 P=1.15
TC Dead	7.0 psf	Rep Mbr Bnd	1.15
BC Live	.0 psf	O.C.Spacing	2- 0- 0
BC Dead	10.0 psf	Design Spec	UBC-97
TOTAL	47.0 psf	Defl Ratio:	L/240 TC: L/240